

Response Spectrum Modal Analysis of Buildings using Spreadsheets

Ravi Kant Mittal¹, P. Prashanth²

¹Assistant Professor, Department of Civil Engineering
Birla Institute of Technology and Science, Pilani-333031, India,
²Formerly Graduate Students, Department of Civil Engineering
Birla Institute of Technology and Science, Pilani-333031, India

ABSTRACT: Seismic analysis of any structure is now mandatory requirement for design of structures. These analyses are very time consuming and tedious. Problem is to be analyzed as per country code only (e.g. IS 1893-2002 for India) in which building is situated. An attempt has been made to make these analyses simple using spread sheets. Spread sheet has been prepared for analysis of structure by response spectrum analysis using SRSS method and CQC method.

Keywords: Earthquake analysis, Response spectrum analysis, Modal combination, SRSS, CQC.

I. INTRODUCTION

Understanding and mastering the seismic analysis of structures is a challenging yet essential element in the field of civil engineering [1-2]. Use of spreadsheets in Civil engineering is reported by various authors [3-7]. In this paper, an Excel-based analysis tool is presented, which attempts to address some of the challenges associated with the problem. The basic principle of the approach is anchored in the need and desire to empower the designer to explore effects of the various parameters on the response of the structure in an environment designed to generate rigorous solutions. The paper also explores the benefits of coupling programming and spreadsheet calculations.

II. RESPONSE SPECTRUM MODAL ANALYSIS OF BUILDINGS USING IS 1893(PART1)-2002

As per IS 1893 (part1)-2002 [8], dynamic analysis shall be performed to obtain the design seismic force, and its distribution to different levels along the height of the building and to the various lateral load resisting elements, for the following buildings:

- Regular buildings — Those greater than 40 m in height in Zones IV and V, and those greater than 90 m in height in Zones II and III.
- Irregular buildings - All framed buildings higher than 12 m in Zones IV and V, and those greater than 40 m in height in Zones II and III. Dynamic analysis may be performed by The Response Spectrum Method. Procedure is summarized in following steps.
 - Modal mass (M_k) – Modal mass of the structure subjected to horizontal or vertical as the case may be, ground motion is a part of the total seismic mass of the Structure that is effective in mode k of vibration. The modal mass for a given mode has a unique value, irrespective of scaling of the mode shape.

$$M_k = \frac{\left[\sum w_i \phi_{ik} \right]^2}{g \sum w_i \phi_{ik}^2}$$

Where

g = acceleration due to gravity,

ϕ_{ik} = mode shape coefficient at floor i in mode k

W_i = Seismic weight of floor i.

(b) Modal Participation factor (P_k) – Modal participation factor of mode k of vibration is the amount by which mode k contributes to the overall vibration of the structure under horizontal or vertical earthquake ground motions. Since the amplitudes of 95 percent mode shape can be scaled arbitrarily, the value of this factor depends on the scaling used for the mode shape.

$$P_k = \frac{\sum w_i \phi_{ik}}{\sum w_i \phi_{ik}^2}$$

(c) Design lateral force at each floor in each mode – The peak lateral force (Q_{ik}) at floor i in

Mode k is given by

$$Q_{ik} = A_{hk} \phi_{ik} P_k W_i$$

Where,

A_{hk} = Design horizontal spectrum value using natural period of vibration (T_k) of mode k.

$$= (Z/2)(I/R)(S_a/g)$$

Z= zone factor for the maximum considered earthquake (MCE), Z/2 stands for DBE

I= Importance factor depending upon the functional use of the structures

R= Response Reduction factor

S_a/g = Average response acceleration coefficient for rock or soil sites as given by response spectra and based on appropriate natural periods and damping of the structure

(d) Storey shear forces in each mode – The peak shear force (V_{ik}) acting in storey i in mode k is given by

$$V_{ik} = \square \square Q$$

(e) Storey shear force due to all modes considered – The peak storey shear force (V_i) in storey i due to all modes considered is obtained by combining those due to each mode as per SRSS or CQC methods.

Total width of the building(m)	6
Total breadth of the building(m)	6
Total height of the building(m)	12
Number of Storeys	4
<div style="display: flex; justify-content: space-around; margin-top: 10px;"> Start Clear </div>	
City	
Zone	4
Structure	Important
Soil type	rocky/hardsoil
Lateral load resisting system	steel MRF as per SP6(6)
seismic intensity,Z	0.24
Importance factor,I	1.5
Response reduction factor,R	5
% Damping	5
Multiplying factor	1

Q_{c1}	Q_{c2}	Q_{c3}	Q_{c4}
21.33272	2.51865	54.35947	7.344951
43.69239	2.826667	-31.9922	-15.2732
61.68749	-1.014	-26.0497	42.76274
71.32689	-4.09144	29.18655	-22.6613
0	0	0	0
V_{i1}	V_{i2}	V_{i3}	V_{i4}
198.0395	0.239871	25.50414	12.17318
176.7068	-2.27878	-28.8553	4.828225
133.0144	-5.10545	3.136873	20.1014
71.32689	-4.09144	29.18655	-22.6613
			correction factor
			4.279801
V_1	200.0458	V_{1c}	856.1565
V_2	179.1268	V_{2c}	766.6272
V_3	134.6581	V_{3c}	576.3098
V_4	80.43417	V_{4c}	344.2423

CALCULATIONS			
Fundamental natural period of vibration of the structure with brick infill,			
in the direction of		$T_a=0.09h/vd$	$T_a=0.075h^{0.75}$
	Width	0.484	
	Breadth	0.441	
S_g/g in direction of			
	Width	2.068010798	
	Breadth	2.068010798	
A_h in direction of			
	Width	0.074448389	
	Breadth	0.074448389	

seismic weight= dead weight of the structure+part of imposed load

For calculation of base shear using this CQC method, user has to input frequency ratio matrix in the specified matrix. Using the formulae given in IS 1893, cross modal coefficients are calculated and shown in the form of a matrix shown in orange colour. λ matrix is the base shears calculated for each mode. This λ and λ^T matrices are used for calculating the base shears of the structure. Correction factor is again to be calculated and multiplied to the outputs. Inputs, calculations and results for some input values are shown in the following figures.

Input frequency ratio matrix				
	β_1	β_2	β_3	β_4
β_1	1	2.919	0	0
β_2	0.343	1	0	0
β_3	0	0	0	0
β_4	0	0	0	0
Cross modal coefficient				
	ρ_1	ρ_2	ρ_3	ρ_4
ρ_1	1	0.006857	0	0
ρ_2	0.006876	1	0	0
ρ_3	0	0	0	0
ρ_4	0	0	0	0
	V_{i1}	V_{i2}	V_{i3}	V_{i4}
	198.0395	0.239871	25.50414	12.17318
	176.7068	-2.27878	-28.8553	4.828225
	133.0144	-5.10545	3.136873	20.1014
	71.32689	-4.09144	29.18655	-22.6613

V_{i1}^T	198.0395	176.7068	133.0144	71.32689
V_{i2}^T	0.239871	-2.27878	-5.10545	-4.09144
V_{i3}^T	25.50414	-28.8553	3.136873	29.18655
V_{i4}^T	12.17318	4.828225	20.1014	-22.6613
Base shear (CQC Method)				
V_1	266.3184	V_{1c}	856.1565	
V_2	2.28973	V_{2c}	7.36099	
V_3	38.37948	V_{3c}	123.3818	
V_4	13.1265	V_{4c}	42.19889	
Correction factor			3.214785	

IV. CONCLUSION

In this paper, the process of development and use of a spreadsheet-based tool for the analysis of multi storey frame under seismic loading is documented. In summary, making of excel spreadsheet for seismic analysis of a multi storey frame is found to be fruitful since the calculation time and energy consumed for these analyses using this spreadsheet is negligible when compared to manual calculations.

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